

IX. Friction Factors

A. Yet another review (!)

Each transport problem involves:

τ τ τ τ τ τ

	<u>"balance"</u> conservation equation	transport law
laminar flow	shell momentum balance	constitutive eq. relating τ to dv/dx
turbulent flow	macroscopic momentum balance → Friction factor	Correlation between f as function of Re

B. General definition of f

$$F_k = AKf \quad ; \quad f = \frac{F_k}{AK} \quad (I)$$

Applicable for: • Flow through cylindrical tube • spherical object falling/rising through fluid
• reactor packed with sediment

$F_k \equiv$ drag force on solid

$A \equiv$ "characteristic area" of solid

$K \equiv$ kinetic energy/unit volume of fluid

C. Tube flow (BSL Sect. 6.2) (Also BSLK)

1. Definitions

$A \equiv$ wetted surface = $2\pi RL$

$K \equiv (1/2) \rho \langle v \rangle^2$

$$\therefore F_k \equiv (2\pi RL) \left(\frac{1}{2} \rho \langle v \rangle^2 \right) f \quad (II)$$

(eq. 6.1-2 BSLK)

Lol

2. Macroscopic momentum balance on fluid in tube

9.2

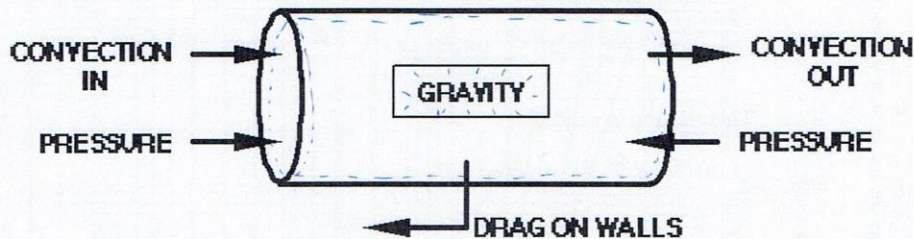
Recall can use macroscopic balance only if either

1) Inturbulence, $\langle v \rangle$ is nearly un. form across the tube

or

2) we don't care how v_z varies with r . Only interested in average, $\langle v_z \rangle$

For turbulent flow, condition (1) applies reasonably well. More fundamentally, condition (2) applies: we don't care how v_z varies across tube; we only want to know drag as function of average v_z . It's understood in what follows that "v" means average velocity, $\langle v_z \rangle$.)



Define system as interior of pipe.

Terms in momentum balance:

convection in: $(\pi R^2) [\rho v^2]_{z=0}$
area

convection out: $(\pi R^2) [\rho v^2]_{z=L}$
 (for incompressible fluids, these terms cancel)

pressure at $Z = 0$: $(\pi R^2) p_0$

pressure at $Z = L$: $(\pi R^2) (-p_L)$
 (negative because pressure at $Z=L$ acts in negative z direction)

body force (gravity): $(\pi R^2 L) \rho g \cos \beta$
volume

(combine gravity and pressure into $(\pi R^2) (P_0 - P_L)$
 $\equiv (\pi R^2) \Delta P$)

drag force on walls: $-F_k$ $(2\pi RL \tau_{r=R})$
 (negative because F_k is defined as positive if momentum leaves system)

No accumulation (acceleration) at steady state

Momentum balance:

$$\Delta P \pi R^2 - F_k = 0; \text{ or } F_k = \Delta P \pi R^2$$

$$F_k = A \cdot k \cdot f \quad + \quad F_k = \frac{\Delta P \pi D^2}{4}$$

9.3

combine two equations for F_k from sections 1 and 2 above -->

$$f = \frac{1}{4} \left(\frac{D}{L} \right) \frac{\Delta P}{\frac{1}{2} \rho v^2}$$

(BSL ^k eq. 6.1-4)

3. Correlations for f

dimensional analysis says $f = f(\text{Re, tube shape})$

"tube shape" means

- cross-sectional shape
- roughness of pipe wall (k)

This relation is valid if

a) L far from entrance

or
b) $L/D \gg 1$

Basic correlation is BSL Fig. 6.2-2

this chart represents transport law

for turbulent flow of Newtonian fluids in pipes;

Eq. 6.1-4 represents the conservation equation or 'balance'

a. for $\text{Re} < 2100$

$f = \frac{16}{\text{Re}}$ (Fanning friction factor); Tube roughness is not important.
The chart (Fig. 6.2-2) is only valid for cylindrical pipe shapes

b. for $\text{Re} > 2100$

for circular tubes:

f depends on ... Reynolds number (Re) and k/D , where
 k = 'height of protuberance'. ΔP greatly increased
over laminar flow \hookrightarrow roughness

from Perry + Chilton, "Chemical Engineers' Handbook,"
5th Edition

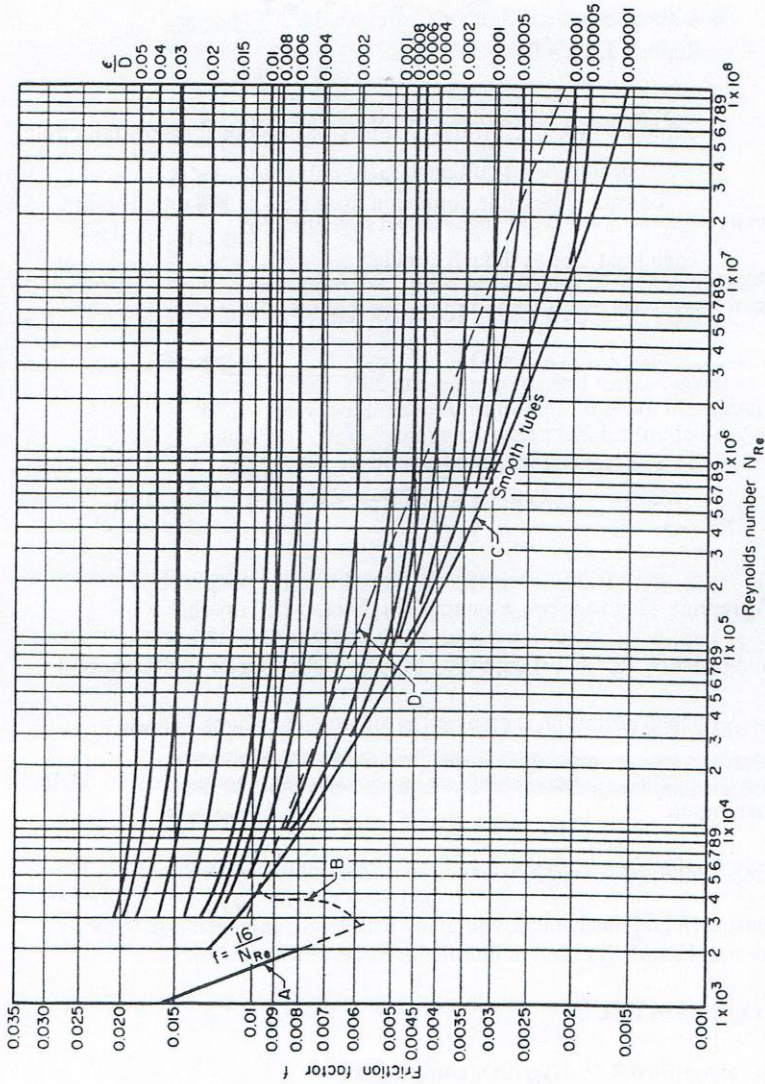


Fig. 5-26. Fanning friction factors. Reynolds number $N_{Re} = DV_0/\mu$, where D = pipe diameter, ft.; V = velocity, ft./sec.; ρ = fluid density, lb./cu. ft.; μ = fluid viscosity, lb./(ft.)(sec.) = cp./1488. [Based on Moody, *Trans. Am. Soc. Mech. Engrs.*, 66, 671 (1944).]

For rough estimates or checks, the velocity-head concept [Lapple, *Chem. Eng.*, 56(5), 96-104 (1949)] can be applied to the first two forms of Eq. (5-52). The velocity head is $V^2/2g_c = h_v$ and the number of velocity-head losses in straight pipe is $4/L/D$. Typical values of h_v and L/D for 1 velocity-head loss are given in Table 5-9.

For cross sections other than circular of ducts running full or for open channels when the variation in depth is negligible, where

Table 5-10. Coordinates for Liquids and Aqueous Solutions
For use with Fig. 5-27

	X	Y	X	Y
Acetaldehyde	6.9	15
Acetic acid, 100%	-0.3	3.7	3.0	37
Acetic acid, 77%	1.0	4.0	1.1	43
Acetic anhydride	2.6	3.8	3.4	15
Acetone, 100%	0.7	4.3
Acetone, 35%	0.9	3.4
	2.7	3.7

See chart

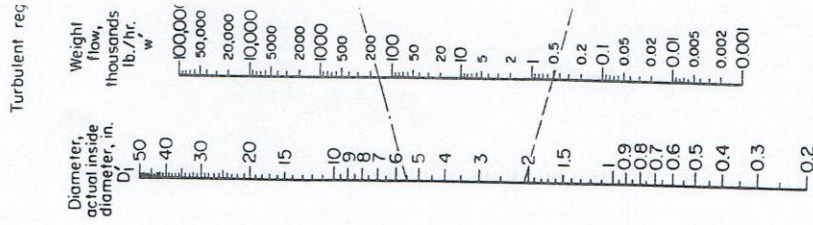


Fig. 5-27. Pipe-flow chart. [General]

the flow is in the turbulent-flow region, the Fanning equation, Eq. (5-52), is replaced by the hydraulic

d. Noncircular tubes: "hydraulic radius approximation"

Define $R_h \equiv S/Z$

(BSLK Eq. 6.2-16)

$S \equiv$ cross-sectional area

$Z \equiv$ perimeter of cross-section

$D_h = 4R_h = 4 \cdot \frac{S}{Z}$

Replace D by $4 R_h$

Compute $\langle v \rangle = \frac{Q}{S}$ with S for true cross-sectional shape (not circle)

beware of possible confusion from BSL p. 133 on this point

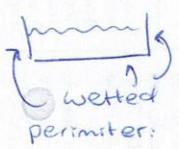
BSLK p. 168

Then treat tube as though circular

NOTE: valid only for turbulent flow

example: narrow slit 

another example: canal



i. For rectangular slit or natural fracture,

$R_h = \frac{2B \cdot W}{2W + 4B}$ where $2B \approx 0 \rightarrow R_h = B$ and $D_h = 4R_h$

4. Warning: There are two common "friction factors" in use

- "Fanning friction factor" differs from "Moody friction factor" by factor of 4
- Beware of which definition is assumed in any reference book you use
- BSL uses "Fanning friction factor", and so do we in this course; has $f = 16/Re$ in laminar region
- "Moody friction factor" charts have $f = 64/Re$ in laminar region
- most civil and mechanical, and many petroleum, engineers use "Moody friction factor"; it's used in Petroleum Engineers handbook

5. examples in BSL

a. example 6.2-1: Given Q, compute $\Delta P/L$

9.7

b. example 6.2-2. Given $\Delta P/L$, compute Q .

Problem: Re is function of unknown $\langle v \rangle$

Method O: trial and error (not in BSL; ^(1,2 OK) taught in CE 319)

- guess $\langle v \rangle$
- compute Re given $\langle v \rangle$ from definition of Re
- determine $f(Re)$ from Fig. 6.2-2 (transport law)
- compute $\langle v \rangle$ from f using eq. 6.1-4 (conservation eq.)
- recompute Re , f , $\langle v \rangle$, etc., until \exists no further changes; at this point solution satisfies both conservation equation and transport law

Warning:

The particular trial-and-error numerical method given above as "Method O" is called "successive substitution." It is not in general an effective numerical method, though it works OK for friction-factor problems.

Don't worry about solution methods in BSL.

6. final notes

Note that equation 6.2-12 ("Blasius eq."), also written on Fig. 6.2-2, is valid only for extremely smooth tubes for $2100 < Re < 100,000$. This can be confusing because this same equation is written on Fig. 6.2-2. To repeat, it is valid only for hydraulically smooth tubes.

In fact, Eqs. 6.2-12 to 6.2-¹⁴ all apply only under limited conditions. if you want an analytical approximation to $f(Re)$ valid for all Re (i.e., usable without knowing Re in advance), then use the equation from Churchill, above.

Don't sweat theoretical discussion of this equation in middle of BSL³ pp. 180-81. (BSLK 165-166)

Example 6.2-2 reworked by trial-and-error method.

Determine the flow rate, in kilograms per hour, of water at 20°C through a 305 m length of horizontal 20.32 cm pipe (internal diameter 20.27 cm) under a differential pressure of 20.68 kPa. For such a pipe use Fig. 6.2-2 and assume that $k/D = \frac{2.3}{2.3} \times 10^{-4}$.

Method 0

guess $\langle v \rangle$

determine Re ($Re = \frac{D \langle v \rangle \rho}{\mu}$)

Find f using Fig. 6.2-2

Transport law

compute $\langle v \rangle$ from eq. 6.1-4

Conservation Eq.

$$f = \frac{1}{4} \left(\frac{D}{L} \right) \left(\frac{\rho_0 - \rho_L}{\frac{1}{2} \rho \langle v \rangle^2} \right)$$

rearranging, we get

$$\langle v \rangle = \left[\frac{1}{4} \left(\frac{D}{L} \right) \frac{\rho_0 - \rho_L}{\frac{1}{2} \rho f} \right]^{1/2}$$

1) guess $\langle v \rangle = 0.05 \text{ m/s}$

9.10

$$Re = \frac{D \langle v \rangle \rho}{\mu} = \frac{(0.2027 \text{ m})(0.05 \text{ m/s})(1000 \text{ kg/m}^3)}{0.00103 \text{ Pa}\cdot\text{s}}$$

$$= 1.97 \cdot 10^5 \langle v \rangle = 9850$$

$$f \approx 0.0077 \text{ (from Fig 6.2-2)}$$

$$2) \text{ calculate } \langle v \rangle = \left[\frac{1/D}{4(L) \rho f} (P_0 - P_2) \right]^{1/2} = \left[\frac{1}{4 \cdot 3.05 \text{ m} \cdot \frac{1}{2} (1000 \frac{\text{kg}}{\text{m}^3}) f} (20680 \text{ Pa}) \right]^{1/2}$$

$$= (6.87 \cdot 10^{-3} / f)^{1/2} = 0.942 \text{ m/s}$$

$$\rightarrow Re = 1.86 \cdot 10^5 \rightarrow f = 0.0045 \rightarrow \langle v \rangle = 1.23 \text{ m/s}$$

$$\rightarrow = 2.43 \cdot 10^5 = 0.0043 \rightarrow = 1.26$$

$$= 2.47 \cdot 10^5 = 0.0043 \rightarrow \langle v \rangle = 1.26$$

[Note $Re = 2.47 \cdot 10^5$ is approximately

the same result as in BSLK Fig 6.2-3 B

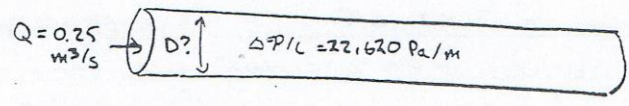
BSLK velocity = 4.01 ft/s = 1.22 m/s. (p. 17)

Same as our answer to within ability

to read chart

Examples of trial-and-error method

To put out the oilfield fires in Kuwait, firefighters used hoses that could deliver up to 4000 gal/min (0.25 m³/s) of seawater. The hoses must extend long distances, so suppose $\Delta P/L$ is limited to 1 psi/ft (22,620 Pa/m). The density and viscosity of water are roughly 1,000 kg/m³ and 0.001 Pa s, respectively. What hose diameter D is required? Assume the roughness factor k/D is 0.004. Note the figure given at the end of the exam.
(25 points)



In this problem, we don't know Re since we don't know D . There are at least 2 ways to solve this problem.

1) Trial + error. $f = \frac{1}{4} \left(\frac{D}{L} \right) \left(\frac{\Delta P}{\rho v^2} \right)$. Since Q is fixed, not $\langle v \rangle$, $\langle v \rangle = Q / (\pi/4) D^2$?

$$\rightarrow f = \frac{1}{4} \frac{D}{L} \frac{\Delta P (\pi/4 D^2)^2}{\frac{1}{2} \rho Q^2} = \frac{1}{4} \frac{\Delta P (\pi/4)^2 D^5}{L \frac{1}{2} \rho Q^2} = \frac{1}{4} (22,620) \frac{(\pi/4)^2 D^5}{(\frac{1}{2})(1000)(0.25)^2} = 111.6 \frac{D^5}{\text{I}}$$

$$Re = \frac{D \langle v \rangle \rho}{\mu} = \frac{D Q \rho}{\mu \frac{\pi}{4} D^2} = \frac{Q \rho 4}{\mu \pi D} = \frac{(0.25)(1000)(4)}{(0.001) \pi D} = 3.18 \cdot 10^5 / D \text{ II}$$

Guess $D = 0.1 \text{ m} \rightarrow Re = 3.2 \cdot 10^6$; from chart, $f = 0.0072 = 111.6 D^5 \rightarrow D = 0.145 \text{ m}$
 Guess $D = 0.145 \text{ m} \rightarrow Re = 2.19 \cdot 10^6$; from chart, $f = 0.0072$ again. $\rightarrow D = 0.145 \text{ m}$
 done. $D = 0.145 \text{ m}$ (about $5 \frac{3}{4} \text{ in.}$)

Stonewall has been assigned to design a large pipeline to transport 0.2 m³/s oil products with a pressure gradient of 10 psi/mile (42.8 Pa/m). The pipeline is horizontal. The oil products have density 850 kg/m³ and viscosity 10 cp (0.01 Pa s). Stonewall figures that even without knowing the pipe diameter, a reasonable estimate of roughness is $k/D = 0.01$. What pipe diameter should he recommend?

Without knowing Re , one can't proceed directly. With D unknown, we don't know v or D . $v = Q / (\pi D^2/4) = 0.2 / (\pi D^2/4) = 0.255 / D^2$
 $Re = D v \rho / \mu = 0 [0.255 / D^2] 850 / 0.01 = 21,645 / D$
 $f = \frac{1}{4} \frac{D}{L} \frac{\Delta P}{\frac{1}{2} \rho v^2} = (1/4) D (42.8) / [\frac{1}{2} (850) (0.255 / D^2)^2] = 0.387 D^5$ (eq. 6.1-4)
 From here, one can proceed in several ways.

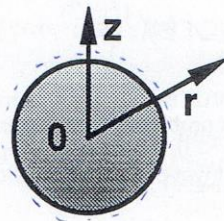
1) Trial + error. Guess $D = 1 \text{ m}$. $Re = 21,645$. From $f(Re)$, $f \approx 0.0105 \approx 0.387 D^5$
 $\rightarrow D = 0.486 \rightarrow Re = 44,500 \rightarrow f(Re) \approx 0.0099 \approx 0.387 D^5 \rightarrow D = 0.480$
 $\rightarrow Re = 45,100 \rightarrow f \approx 0.0099$ again. Done. $D = 0.48 \text{ m}$.

D. flow around spheres (BSL Sect. 6.3)

1. Definitions

$$F_k = A K f \equiv (\pi R^2) (\frac{1}{2} \rho v_\infty^2) f \quad \text{BSLk 6.3-14} \quad \text{(I)}$$

2. macroscopic momentum balance



Note coordinate system is centered on sphere, and z points up; fluid flows up past sphere, which is stationary in this coordinate system.

Define system as solid sphere.

Terms in momentum balance:

drag on sphere: F_k
(positive, because drag pushes sphere in positive z direction)

gravity: $-\frac{4}{3} \pi R^3 \rho_s g$ (gravity pulls down)
(pulls sphere in negative z direction)

In BSLk

buoyancy: $\frac{4}{3} \pi R^3 \rho g$

(ρ without subscript refers to liquid; buoyancy pushes sphere in positive z direction)

there is no accumulation (because balance is at steady state) and no convection across system boundaries (i.e., into or out of sphere)

Momentum balance:

$$F_k - \frac{4}{3} \pi R^3 (\rho_s - \rho) g = 0 \quad \text{f steady state} \quad \text{(II)}$$

$$\rightarrow F_k = \frac{4}{3} \pi R^3 (\rho_s - \rho) g \quad \text{f BSLk eq. 6.1-6}$$

combine eqs. I, II -->

$$f = \frac{4}{3} \left(\frac{gD}{v_\infty^2} \right) \left[\frac{\rho_s - \rho_f}{\rho_f} \right] \quad \text{BSLK (BSL eq. 6.1-7)}$$

or

$$v_\infty = \sqrt{\frac{4}{3} g D \left(\frac{\rho_s - \rho_f}{\rho_f} \right)}$$

In either form, this equation represents the conservation equation for this problem; still need transport law
 If $(\rho_s - \rho) < 0$, then simply drop minus sign, and note that fluid flows down past sphere (sphere floats up through fluid)

3. correlation for f

Again, $f = f(\text{Re})$; $\text{Re} \equiv \frac{D v_\infty \rho_f}{\mu_f}$; (ρ and μ refer to fluid)

correlation for $f(\text{Re})$ represents transport law for this problem;

given by Fig 6.3-1

this correlation is for an isolated, smooth sphere in an infinite body of fluid; roughness and spinning of the sphere can greatly affect its behavior, as can interference from a solid wall or other spheres; for further discussion of spinning and roughness, see R. G. Watts and A. T. Bahill, *Keep Your Eye on the Ball: The Science and Folklore of Baseball*, W. H. Freeman and Co., New York, 1990.

9.15

4. Solving for unknown Re

This is the usual case in problems of falling spheres: unknown is usually D , v_{∞} , ρ or μ . Can't compute Re without knowing answer to problem

cf. BSL Example 6.3-1: don't know D

Method O: trial and error (not in BSL; taught in CE 319)

- guess D
- compute Re from assumed D using definition of Re
- Determine f from calculated Re using Fig. 6.3-1
(transport law)
- recompute D from eq. 6.1-7
(conservation of momentum)
- repeat calculation of Re, f , D , etc., until \exists no further changes -
at this point, solution satisfies both conservation equation and
transport law

BSUIC
Example 6.3-1. (BSL) by trial and error method

Given: $\rho = 1.59 \text{ gcm}^{-3} = 1590 \text{ kgm}^{-3}$
 $\rho_s = 2.62 \text{ gcm}^{-3} = 2620 \text{ kgm}^{-3}$
 $\mu = 9.58 \text{ millipoise} = 9.58 \times 10^{-4} \text{ kgm}^{-1}\text{s}^{-1}$
 $T = 20^\circ\text{C}$
 $V_\infty = 65 \text{ cms}^{-1} = 0.65\text{ms}^{-1}$
 $D = ?$

$$f = \frac{4}{3} \frac{gD}{V_\infty^2} \left(\frac{\rho_s - \rho}{\rho} \right) \dots\dots\dots 1$$

$$\text{Re} = \frac{D\rho V_\infty}{\mu} \dots\dots\dots 2$$

Solution procedure:

Guess D (arbitrary), Solve for Re, read f from chart (Fig. 6.3-1 BSL page 192), Recalculate D, then repeat calculation of Re, f and D until D equals guessed value.

steps : Substituting values into equations 1 and 2 above gives:

$$f = \frac{4}{3} \frac{9.81}{0.65^2} \left(\frac{2620 - 1590}{1590} \right) D = 20.055 D$$

$$\text{Re} = \frac{D \times 1590 \times 0.65}{9.58 \times 10^{-4}} \approx 1078810 D$$

Guessing $D = 0.04\text{m}$
 $\text{Re} = 1078810 \times 0.04 = 43152.4$ and $f \approx 0.48$

Recalculating D twice gives:

$D = 0.48 / 20.055 \approx 0.0239\text{m}$ and
 $\text{Re} = 1078810 \times 0.0239 = 25820$, giving $f \approx 0.44$ from chart.

$D = 0.44 / 20.055 \approx 0.0219\text{m}$ and
 $\text{Re} = 1078810 \times 0.0214 = 23098$, giving $f \approx 0.44$ from chart.

Since the value of f is same, **$D = 0.0219\text{m} = 2.19\text{cm}$**

An example of trial and error in falling-sphere problem

Safety experts warn gun owners that firing into the air can be dangerous, because the bullet can fall to earth with enough velocity to hurt someone. Suppose a bullet were a sphere of lead (density 11,300 kg/m³) weighing 1/2 ounce (0.0156 kg). What would be the terminal velocity of such a sphere falling from a great height through air (with viscosity $\mu = 1.8 \times 10^{-5}$ Pa s and density $\rho = 1.3$ kg/m³)? (20 points)

We don't know v_t or Re . \therefore some sort of special or trial-and-error solution is needed. 3 several possible ways to proceed.

i) trial + error: $Re = Dv\rho/\mu$; What is D? $\frac{4}{3}\pi R^3 \rho = 0.0156$.

$$\frac{4}{3}\pi R^3 (11,300) = 0.0156 \rightarrow R = 0.00691 \text{ m}; D = 0.0138 \text{ m}$$
$$Re = \frac{(0.0138) v (1.3)}{(1.8 \cdot 10^{-5})} = 997 v$$

$$f = \frac{4}{3} g D \frac{1}{v^2} \left(\frac{\rho_s - \rho}{\rho} \right) = \frac{4}{3} (9.8) (0.0138) \frac{1}{v^2} \left(\frac{11,300 - 1.3}{1.3} \right) = 1567 / v^2$$

guess $v = 1 \text{ m/s}$. $Re = 997$. $f \approx 0.45 > 1567/v^2 \rightarrow v = 59.0 \text{ m/s}$
 \leftarrow from Fig 6.3-1

guess $v = 59.0$. $Re = 5.9 \cdot 10^4$. from Fig 6.3-1, $f \approx 0.49$ $v = \sqrt{\frac{1567}{0.49}} = 56.6$

guess $v = 56.5$ $Re = 5.64 \cdot 10^4$. " $f = 0.49$ No change from last iteration; \therefore done. $v = 56 \text{ m/s}$

order steps:

- guess unknown
- calculate Re \leftarrow
- Find $f(Re)$ from chart
- calculate unknown from f
- did unknown change?
- yes:
- no: done

E. packed columns.

1. Applications in Petroleum Engineering and Groundwater Flow

- Unconsolidated / ~~likely~~ ^{lightly} consolidated geological Formation
- Gravel pack around well = "Filter" around well filled with sand particles
- propped fractures
 - unless plugged by fine particles chemical Gunk

not valid for

- Consolidated / cemented geological Formation
- Packing plugged by particles etc.
- Packing of deformable particles

2. Definitions

$$D_p \equiv \text{Particle diameter (if spherical)}$$

(see BSL if particle is not spherical)

$$G_0 \equiv \rho v_0$$

$$v_0 \equiv \text{Darcy velocity} = \frac{Q_t}{A} \rightarrow \text{cross sectional area of pack}$$

(would be written as "u" or Q/A in petroleum engineering)

$$\varepsilon \equiv \text{Porosity}$$
(would be written " ϕ " in petroleum engineering)
$$\rho \equiv \text{Density of fluid if incompressible}$$

(see BSL if fluid is very compressible)

3. correlation for f

Fig. 6.4-2 gives correlation for $f \left(\frac{\epsilon^3}{1-\epsilon} \right)$ (vertical axis) v. Re (horizontal axis)

easier to use Ergun equation since it covers entire extent of chart

Note $Re \equiv \left(\frac{D_p G_0}{\mu} \right) \left(\frac{1}{1-\epsilon} \right) \equiv \frac{\Delta p V_0 \rho}{\mu} \cdot \frac{1}{1-\epsilon}$

(note volumetric flux v_0 in definition of Re instead of velocity)
 For most applications, it's simpler to just use equations in text:

- Don't need to worry about laminar vs. turbulent - eq. 6.4-12 valid for all Re; or
- eq. 6.4-9 valid for $Re \leq 10$ and $\epsilon < 0.5$
- eq. 6.4-11 valid for $Re \geq 1,000$

4. Comparison to Darcy's law - k for sandpack ← as defined by petroleum engineers

For $Re \leq 10$, eq. 6.4-9 applies:

$$v_0 \equiv \frac{Q}{A} = \frac{\Delta P}{L} \frac{\Delta P^2}{150 \mu} \frac{\epsilon^3}{(1-\epsilon)^2}$$

'specific discharge' in soil mechanics

compare to Darcy's law, also valid only at low Re (absence of turbulence):

as used by petroleum engineers

$$\frac{k}{A} \frac{\Delta h}{\Delta L} \quad \frac{Q}{A} = \frac{k}{\mu} \frac{\Delta P}{L}$$

→ $k = \frac{D_p^2 \epsilon^3}{150 (1-\epsilon)^2} \frac{\rho v_0}{\mu}$ permeability

for $Re \equiv \left(\frac{D_p v_0 \rho}{\mu} \frac{1}{1-\epsilon} \right) \leq 10$

(Note this implies a transition to turbulence at much lower flow rate than implied by BOT model)

Why use these correlations for packed beds instead of the BOT model? BOT model requires measured k and ϕ . Eq. 6.4-9 predicts k for given ϵ and D_p .